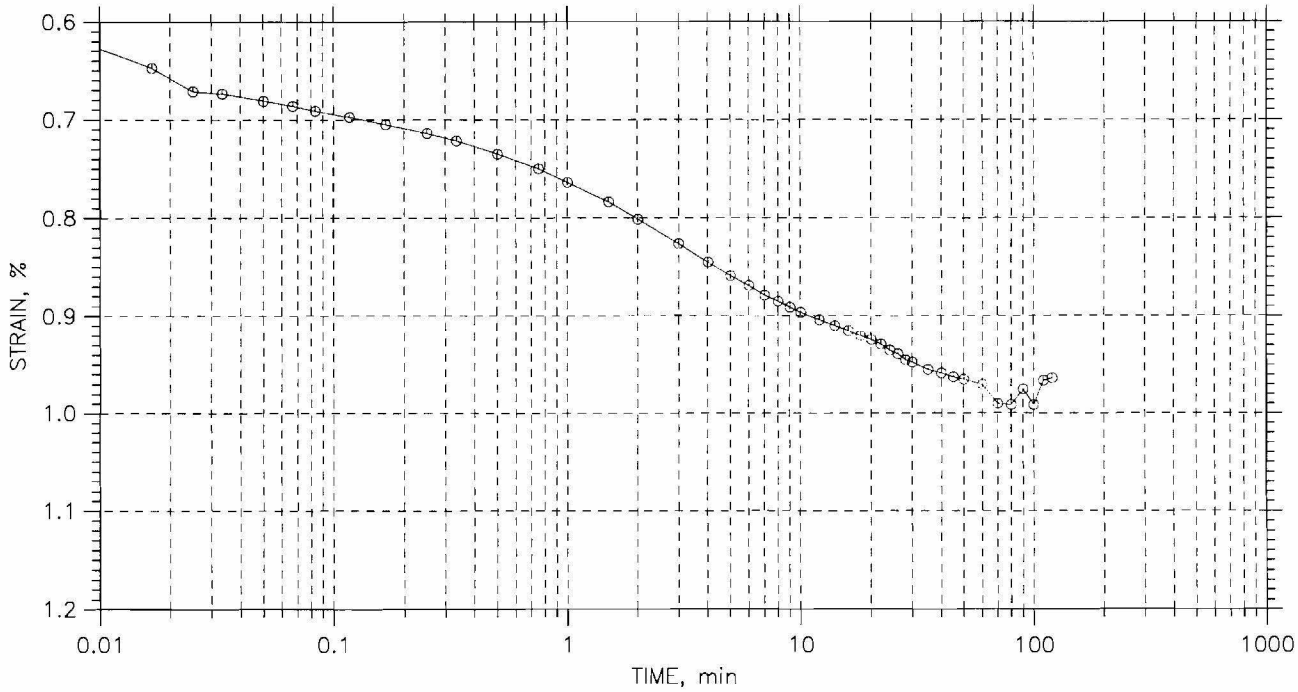
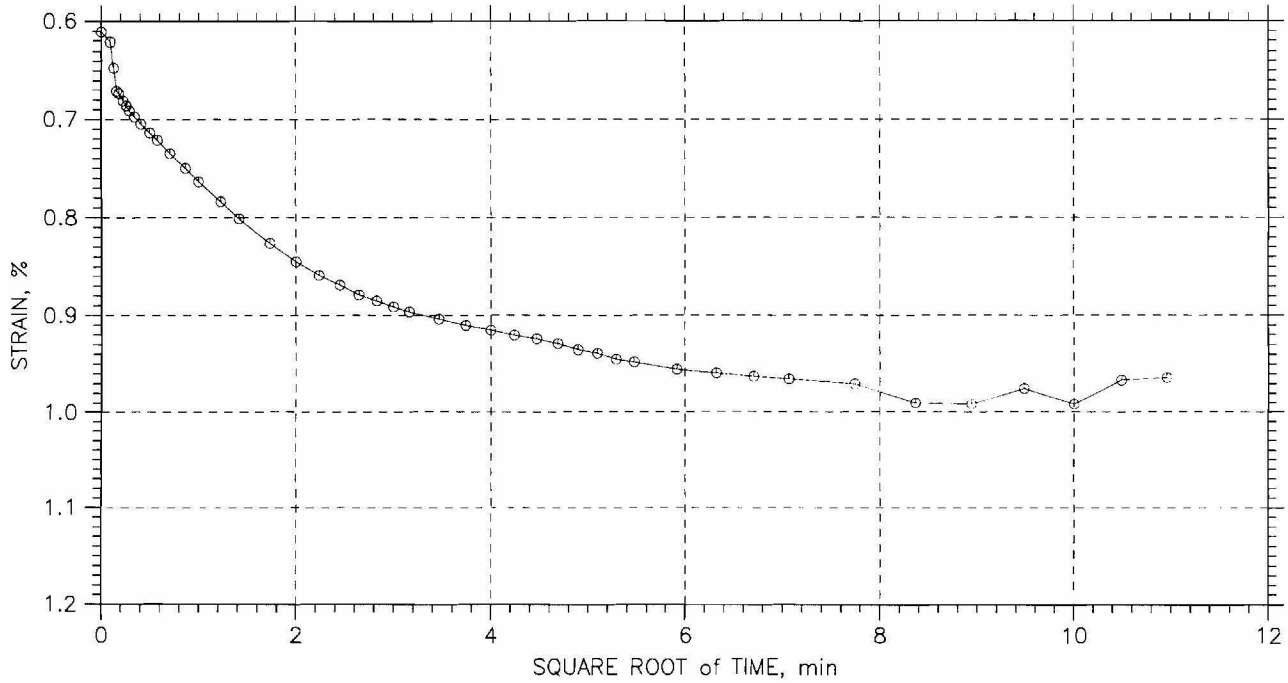
CONSOLIDATION TEST DATA

TIME CURVES

Constant Load Step: 2 of 12

Stress: 0.25 tsf





GeoTesting express	Project: No. 064006	Location: Windham, ME	Project No.: GTX-7278
	Boring No.: B-114	Tested By: md	Checked By: jdt
	Sample No.: ---	Test Date: 02/06/07	Depth: 23-25 ft
	Test No.: C-1	Sample Type: Tube	Elevation: ---
	Description: Moist, gray clay with traces of sand		
	Remarks: System G		

VIL_RESP00699

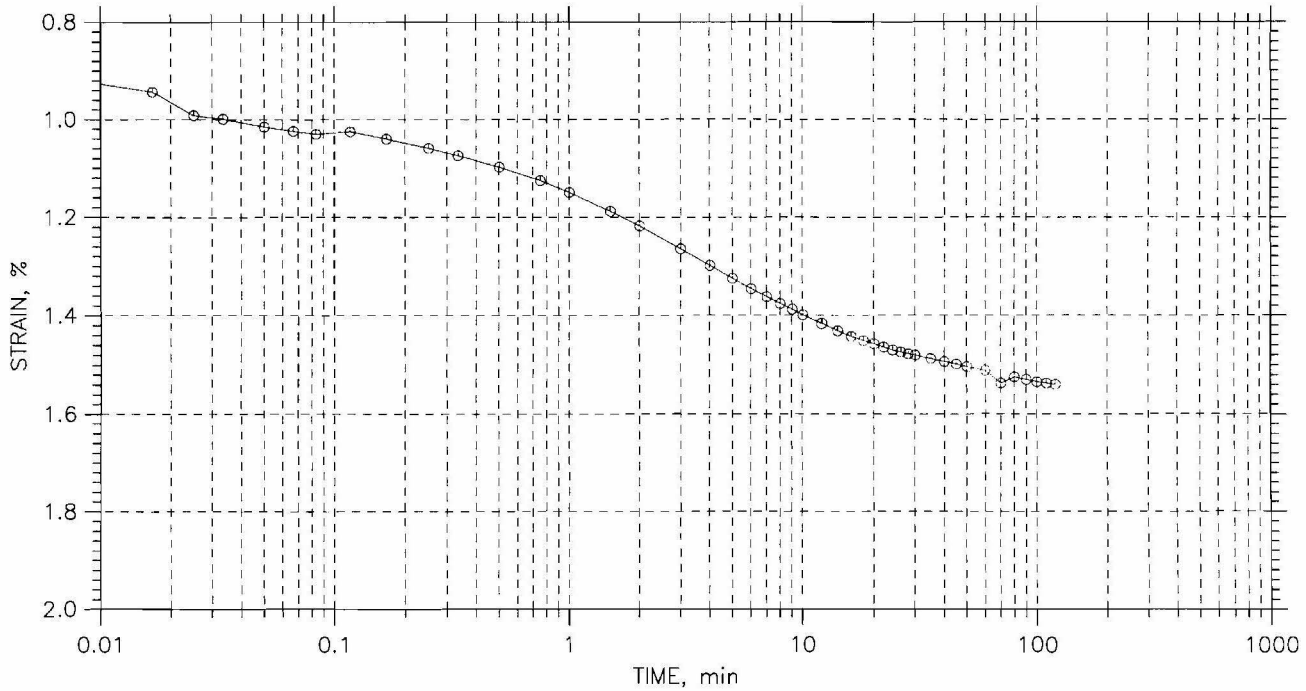
Wed, 14-FEB-2007 13:47:06

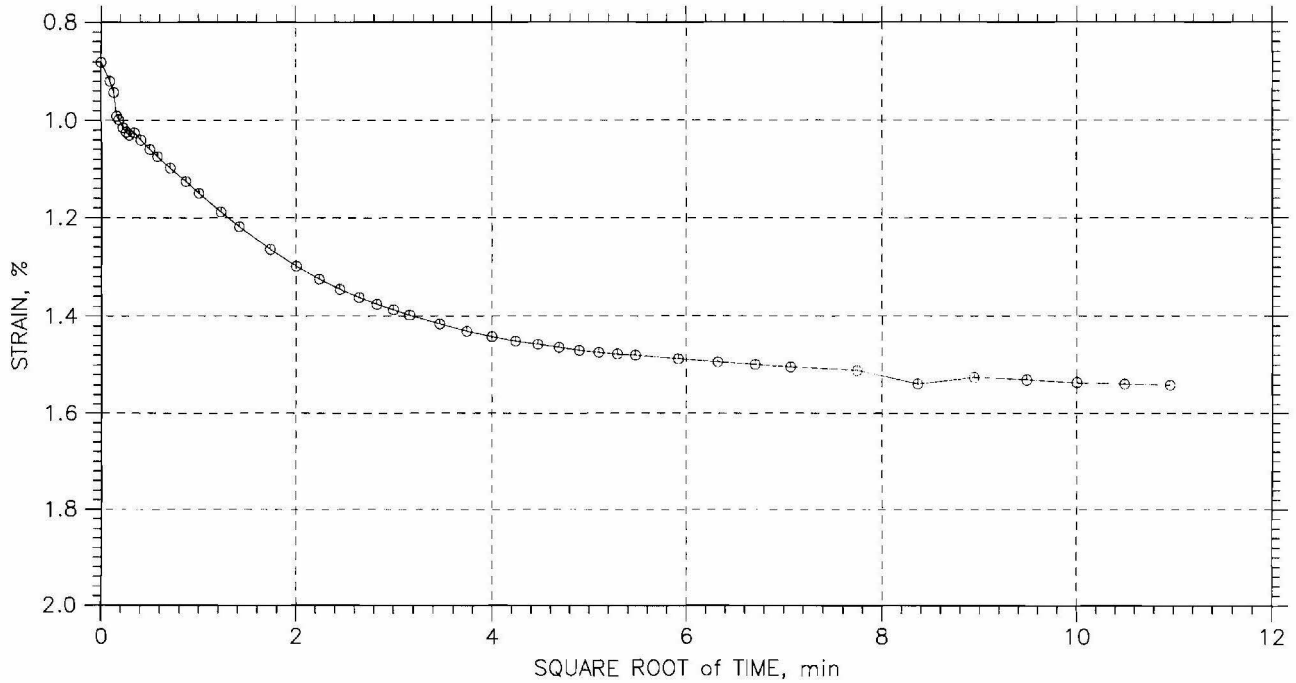
CONSOLIDATION TEST DATA

TIME CURVES

Constant Load Step: 3 of 12

Stress: 0.5 tsf





GeoTesting express	Project: No. 064006	Location: Windham, ME	Project No.: GTX-7278
	Boring No.: B-114	Tested By: md	Checked By: jdt
	Sample No.: ---	Test Date: 02/06/07	Depth: 23-25 ft
	Test No.: C-1	Sample Type: Tube	Elevation: ---
	Description: Moist, gray clay with traces of sand		
	Remarks: System G		

VIL_RESP00700

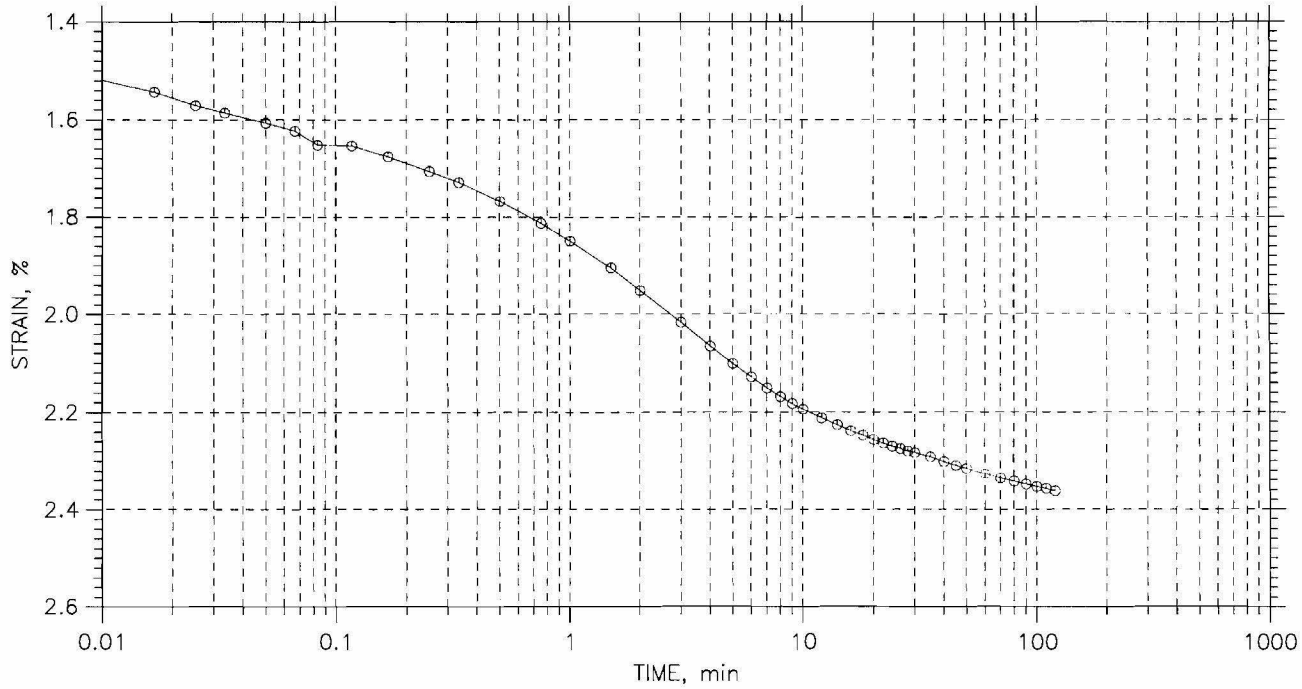
Wed, 14-FEB-2007 13:47:06

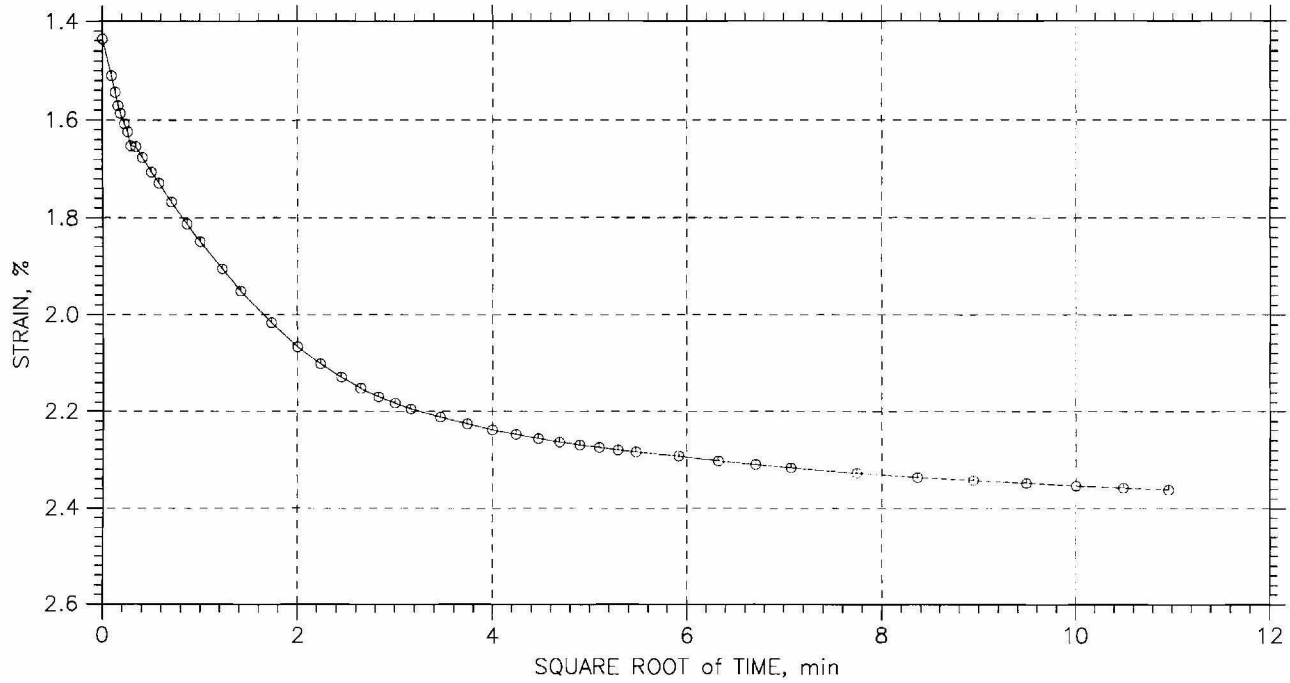
CONSOLIDATION TEST DATA

TIME CURVES

Constant Load Step: 4 of 12

Stress: 1. tsf





GeoTesting express	Project: No. 064006	Location: Windham, ME	Project No.: GTX-7278
	Boring No.: B-114	Tested By: md	Checked By: jdt
	Sample No.: ---	Test Date: 02/06/07	Depth: 23-25 ft
	Test No.: C-1	Sample Type: Tube	Elevation: ---
	Description: Moist, gray clay with traces of sand		
	Remarks: System G		

VIL_RESP00701

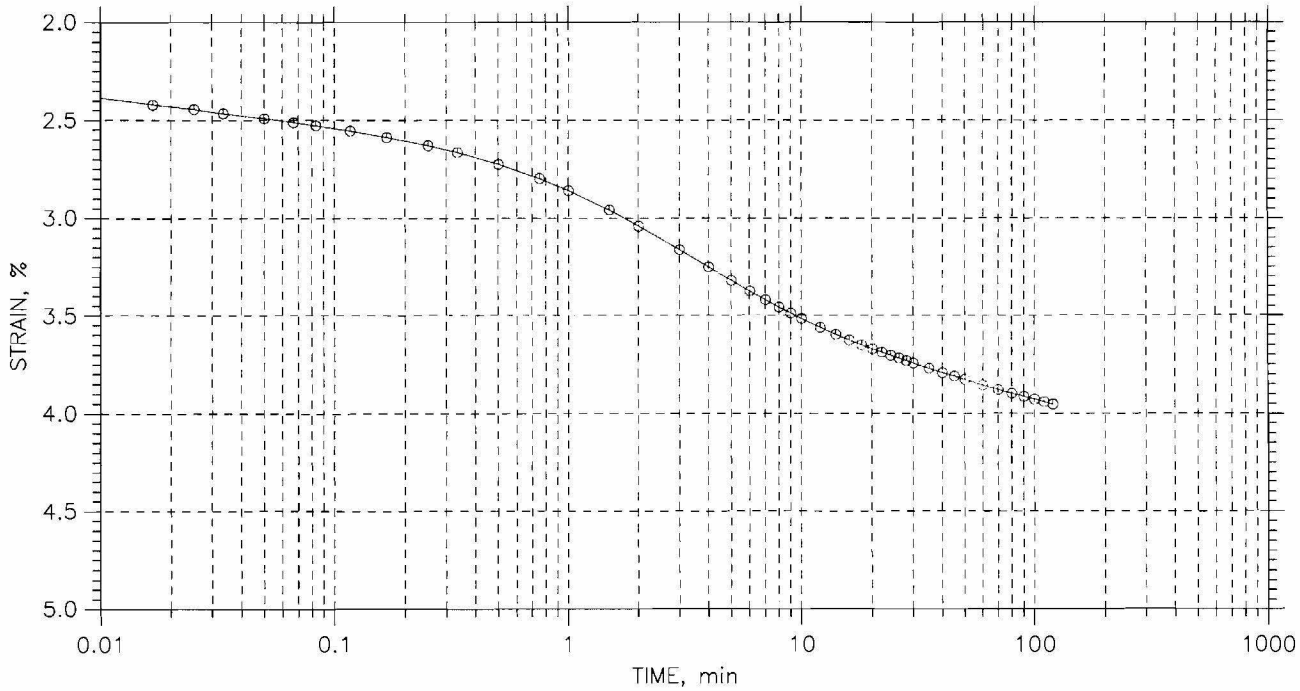
Wed, 14-FEB-2007 13:47:06

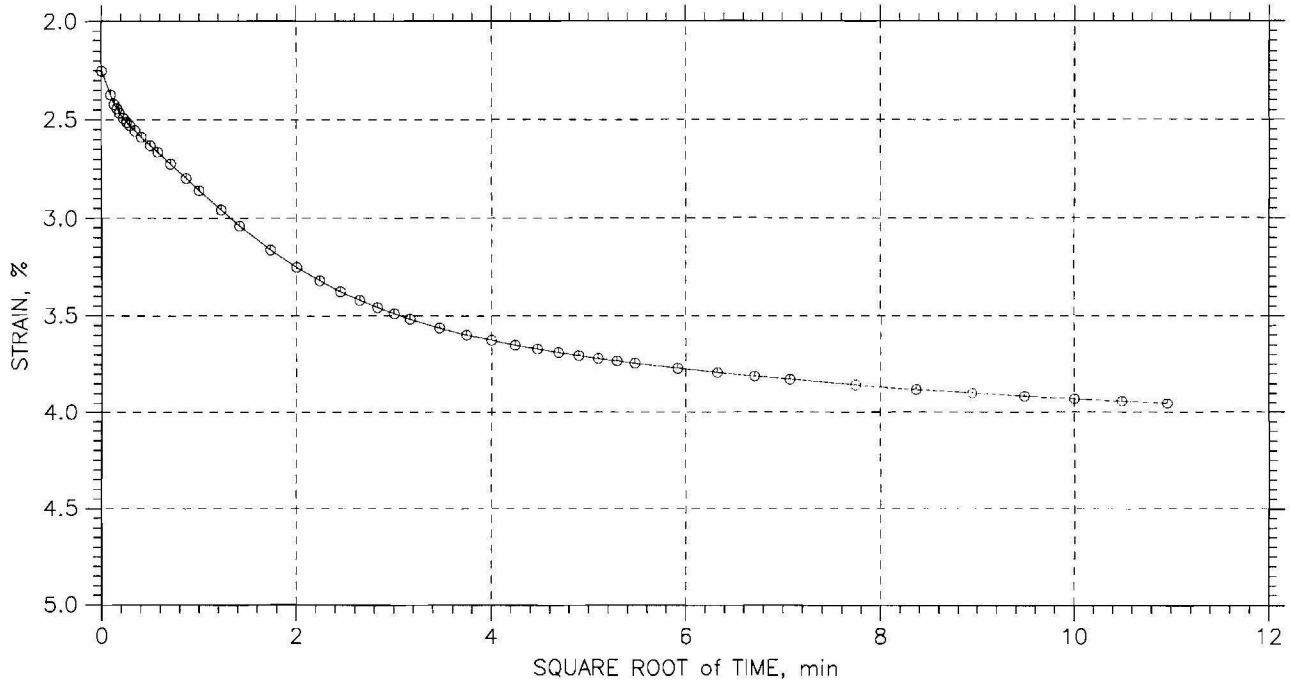
CONSOLIDATION TEST DATA

TIME CURVES

Constant Load Step: 5 of 12

Stress: 2. tsf





GeoTesting express	Project: No. 064006	Location: Windham, ME	Project No.: GTX-7278
	Boring No.: B-114	Tested By: md	Checked By: jdt
	Sample No.: ---	Test Date: 02/06/07	Depth: 23-25 ft
	Test No.: C-1	Sample Type: Tube	Elevation: ---
	Description: Moist, gray clay with traces of sand		
	Remarks: System G		

VIL_RESP00702

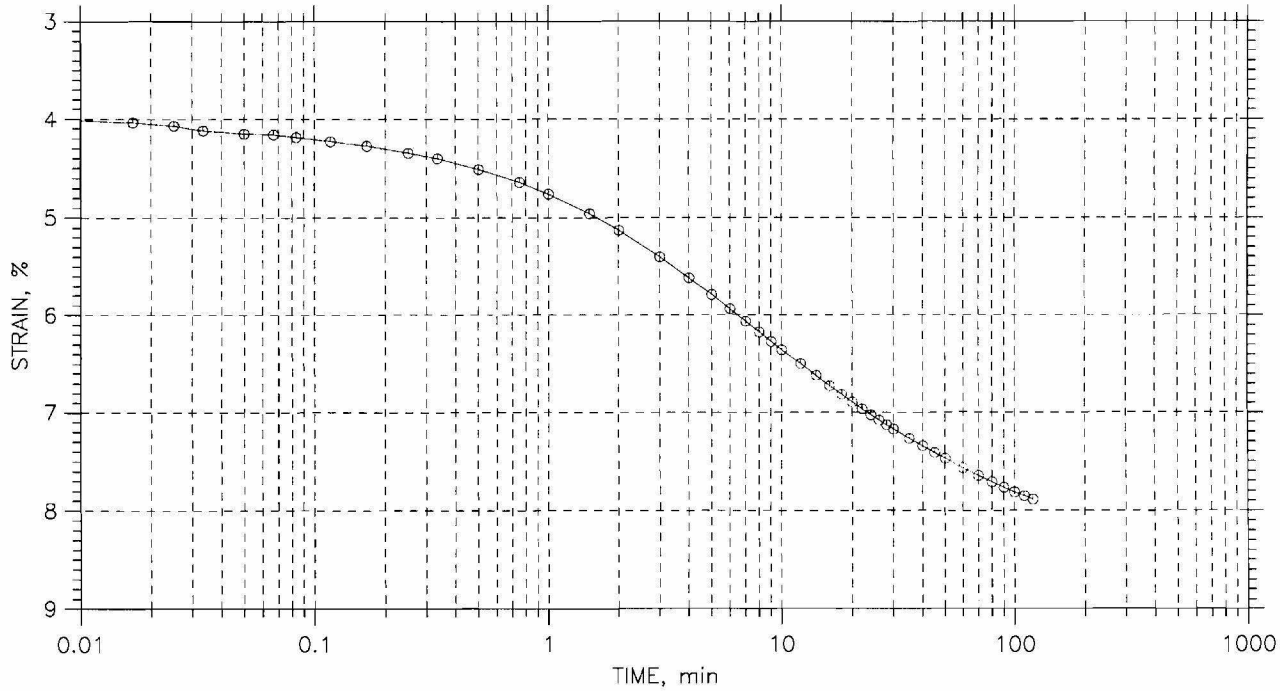
Wed, 14-FEB-2007 13:47:06

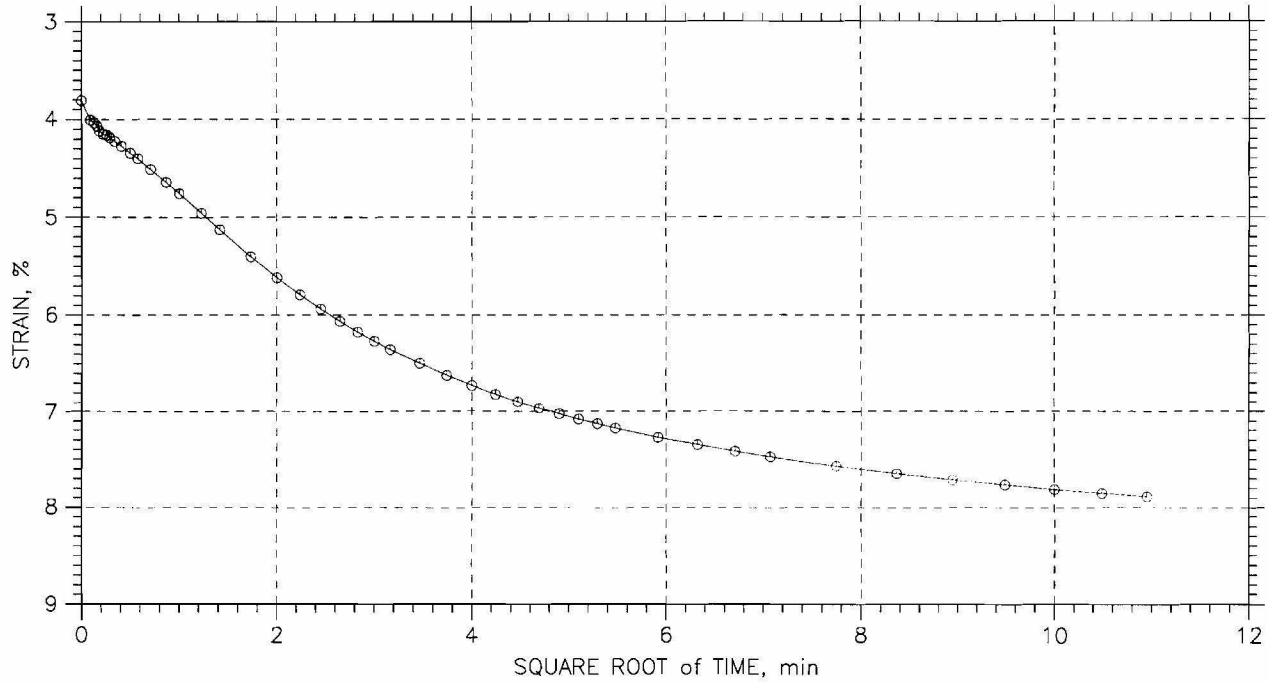
CONSOLIDATION TEST DATA

TIME CURVES

Constant Load Step: 6 of 12

Stress: 4. tsf





GeoTesting express	Project: No. 064006	Location: Windham, ME	Project No.: GTX-7278
	Boring No.: B-114	Tested By: md	Checked By: jdt
	Sample No.: ---	Test Date: 02/06/07	Depth: 23-25 ft
	Test No.: C-1	Sample Type: Tube	Elevation: ---
	Description: Moist, gray clay with traces of sand		
	Remarks: System G		

VIL_RESP00703

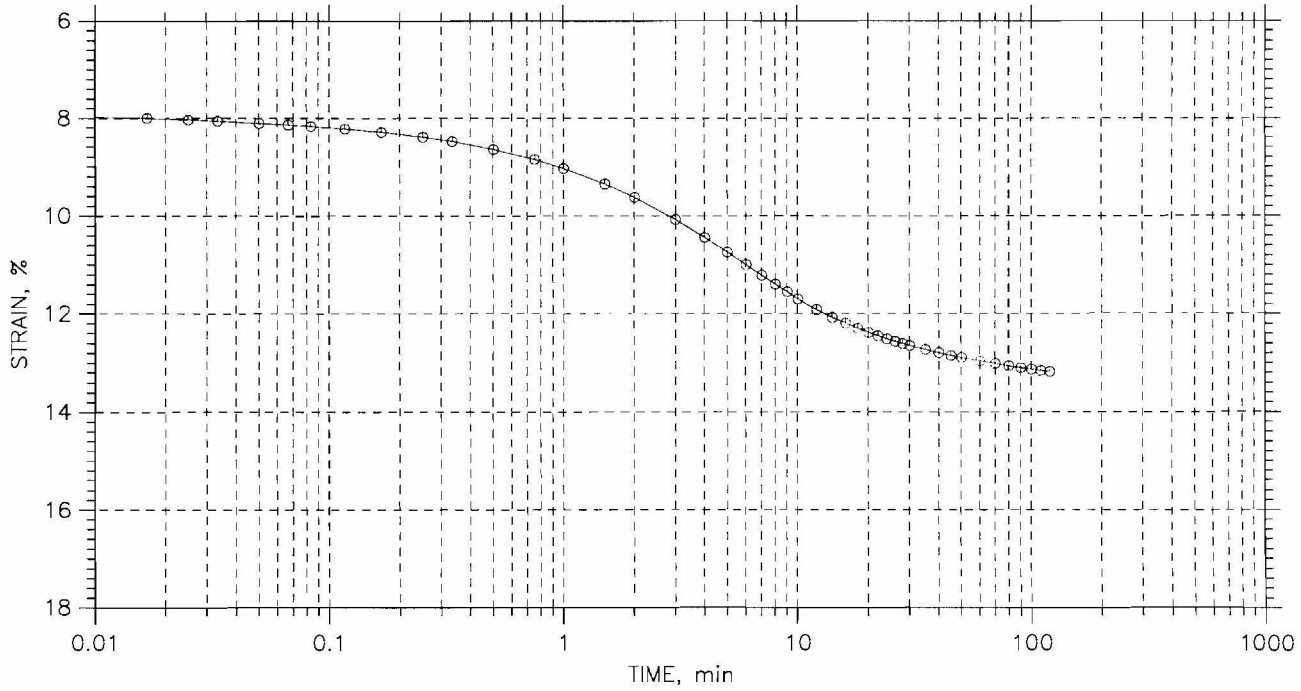
Wed, 14-FEB-2007 13:47:06

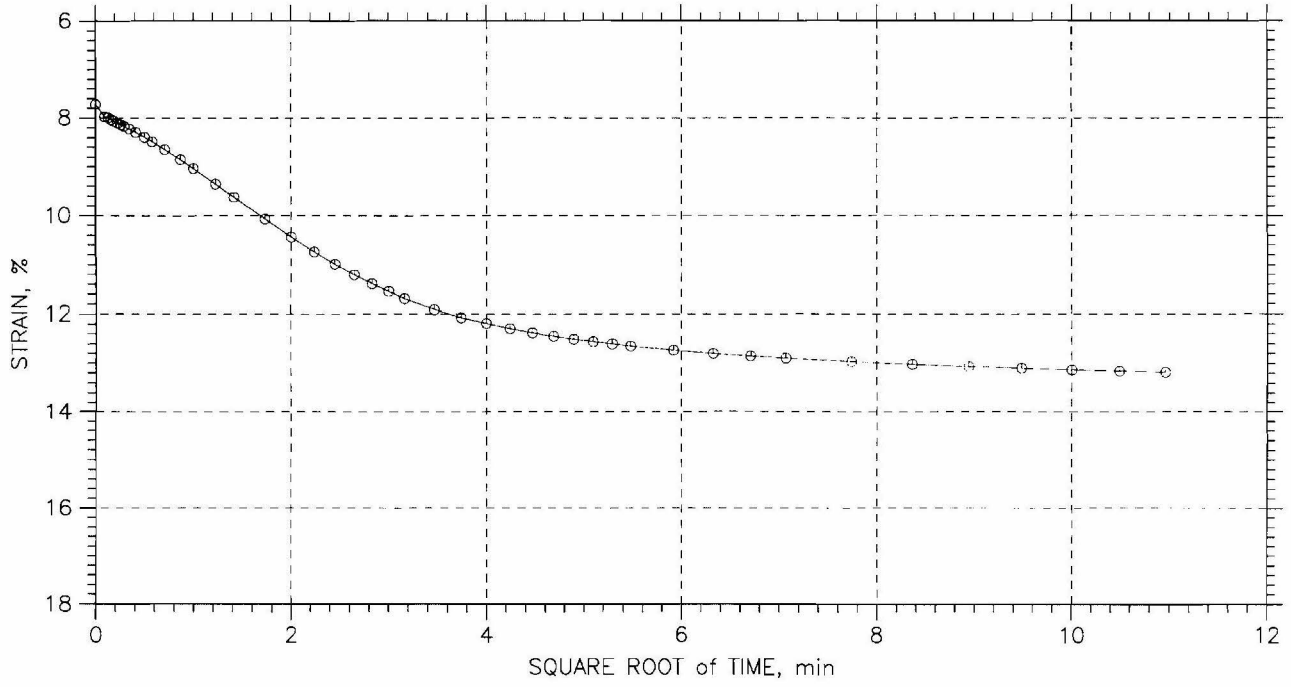
CONSOLIDATION TEST DATA

TIME CURVES

Constant Load Step: 7 of 12

Stress: 8. tsf





GeoTesting express	Project: No. 064006	Location: Windham, ME	Project No.: GTX-7278
	Boring No.: B-114	Tested By: md	Checked By: jdt
	Sample No.: ---	Test Date: 02/06/07	Depth: 23-25 ft
	Test No.: C-1	Sample Type: Tube	Elevation: ---
	Description: Moist, gray clay with traces of sand		
	Remarks: System G		

VIL_RESP00704

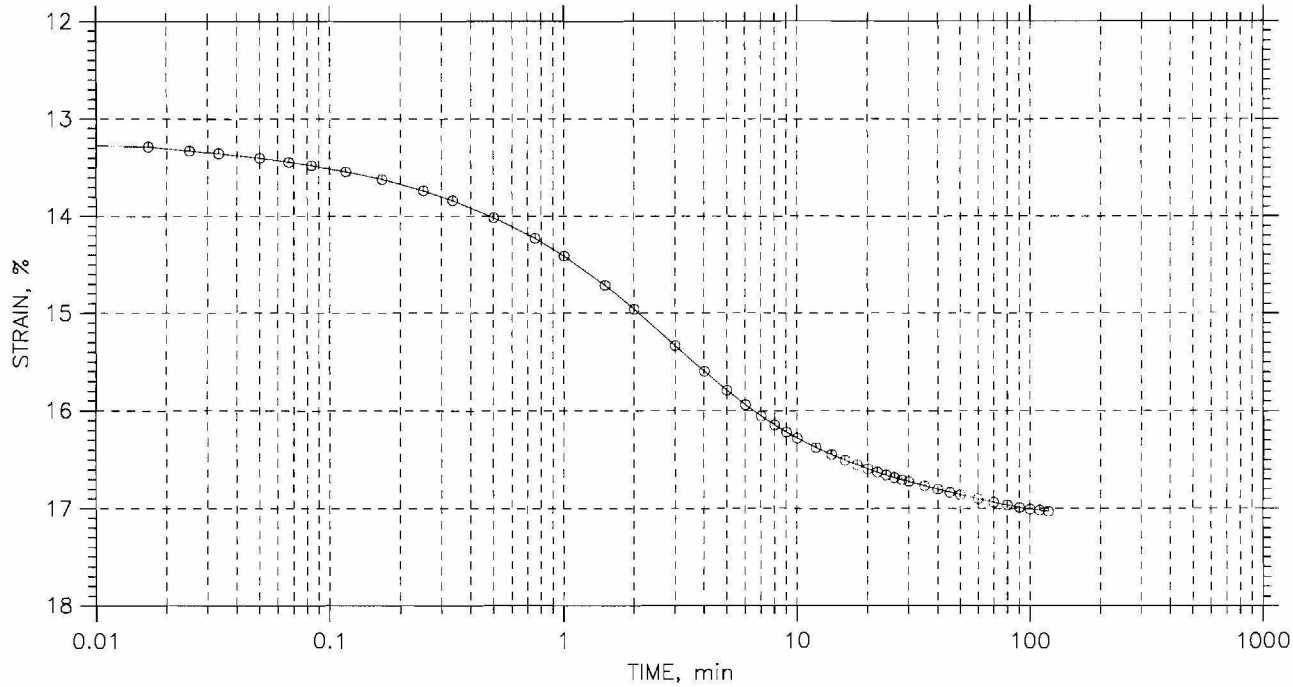
Wed, 14-FEB-2007 13:47:06

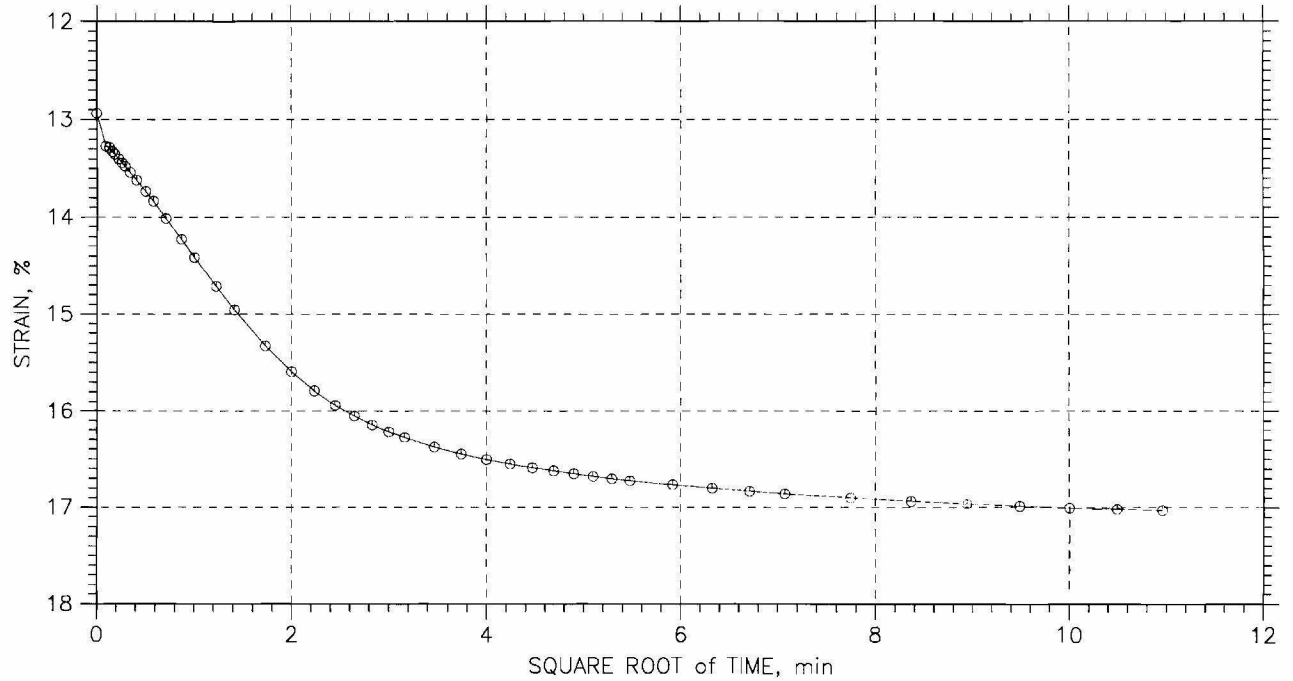
CONSOLIDATION TEST DATA

TIME CURVES

Constant Load Step: 8 of 12

Stress: 16. tsf





GeoTesting express	Project: No. 064006	Location: Windham, ME	Project No.: GTX-7278
	Boring No.: B-114	Tested By: md	Checked By: jdt
	Sample No.: ---	Test Date: 02/06/07	Depth: 23-25 ft
	Test No.: C-1	Sample Type: Tube	Elevation: ---
	Description: Moist, gray clay with traces of sand		
	Remarks: System G		

VIL_RESP00705

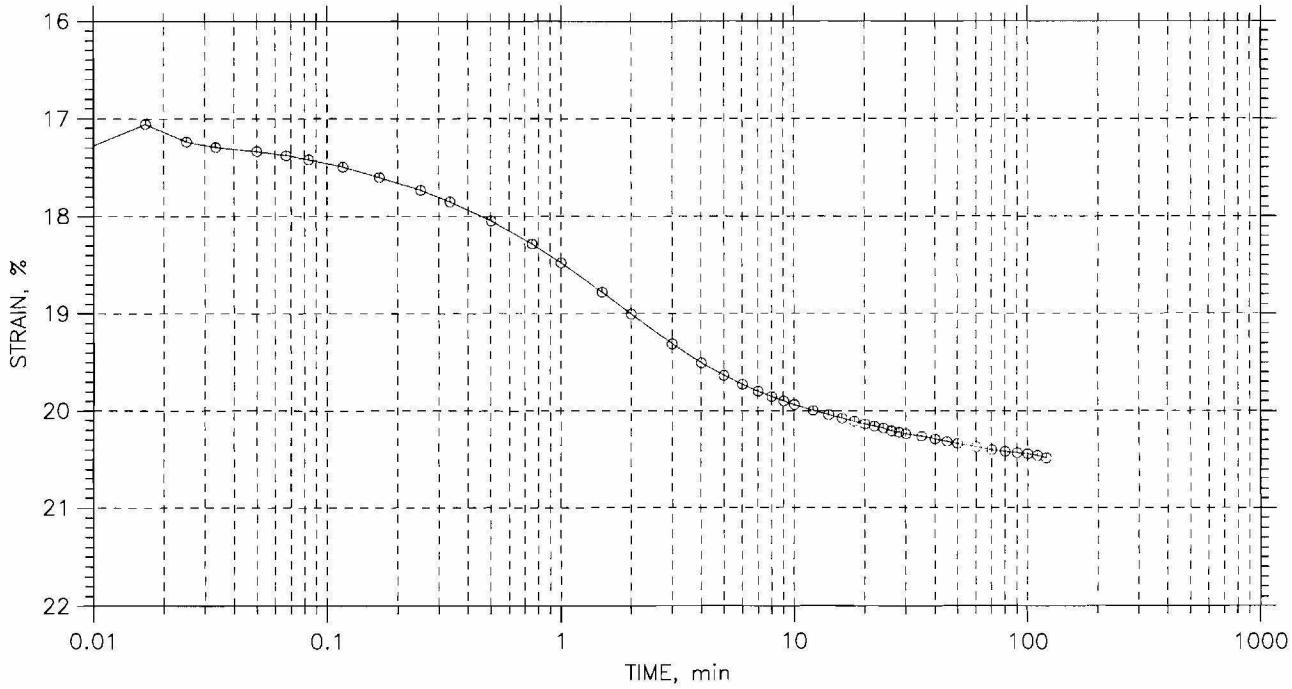
Wed, 14-FEB-2007 13:47:06

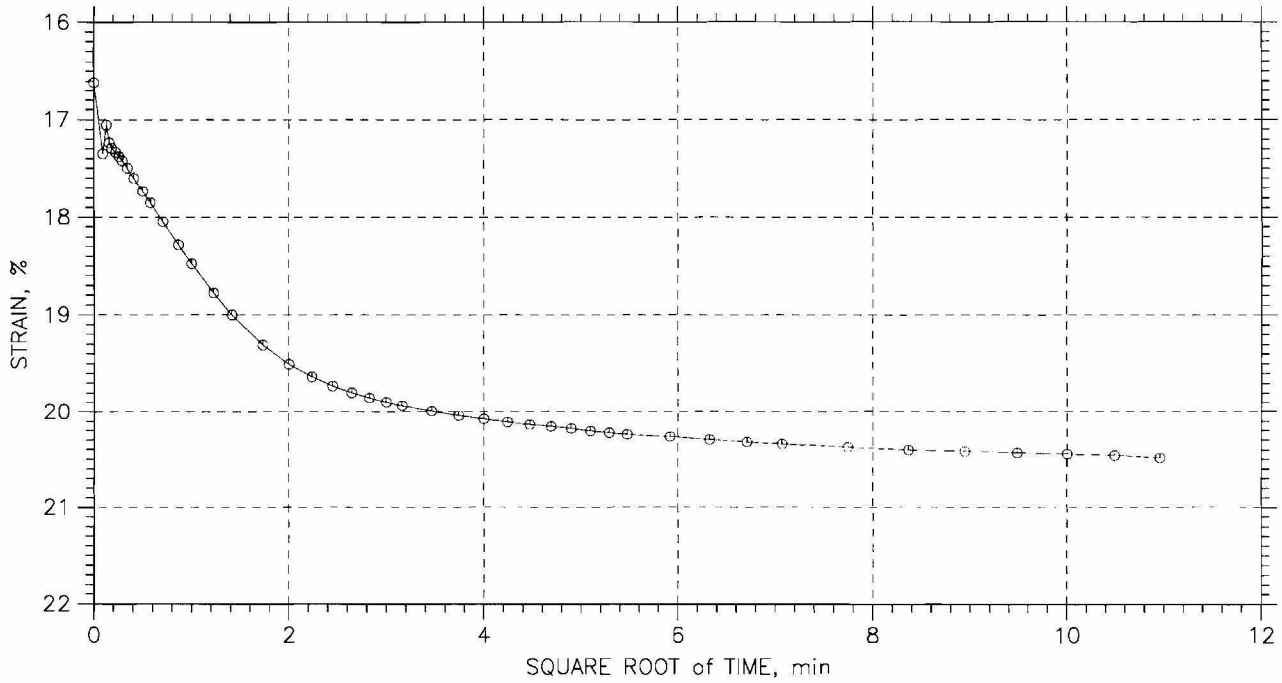
CONSOLIDATION TEST DATA

TIME CURVES

Constant Load Step: 9 of 12

Stress: 32. tsf





GeoTesting express	Project: No. 064006	Location: Windham, ME	Project No.: GTX-7278
	Boring No.: B-114	Tested By: md	Checked By: jdt
	Sample No.: ---	Test Date: 02/06/07	Depth: 23-25 ft
	Test No.: C-1	Sample Type: Tube	Elevation: ---
	Description: Moist, gray clay with traces of sand		
	Remarks: System G		

VIL_RESP00706

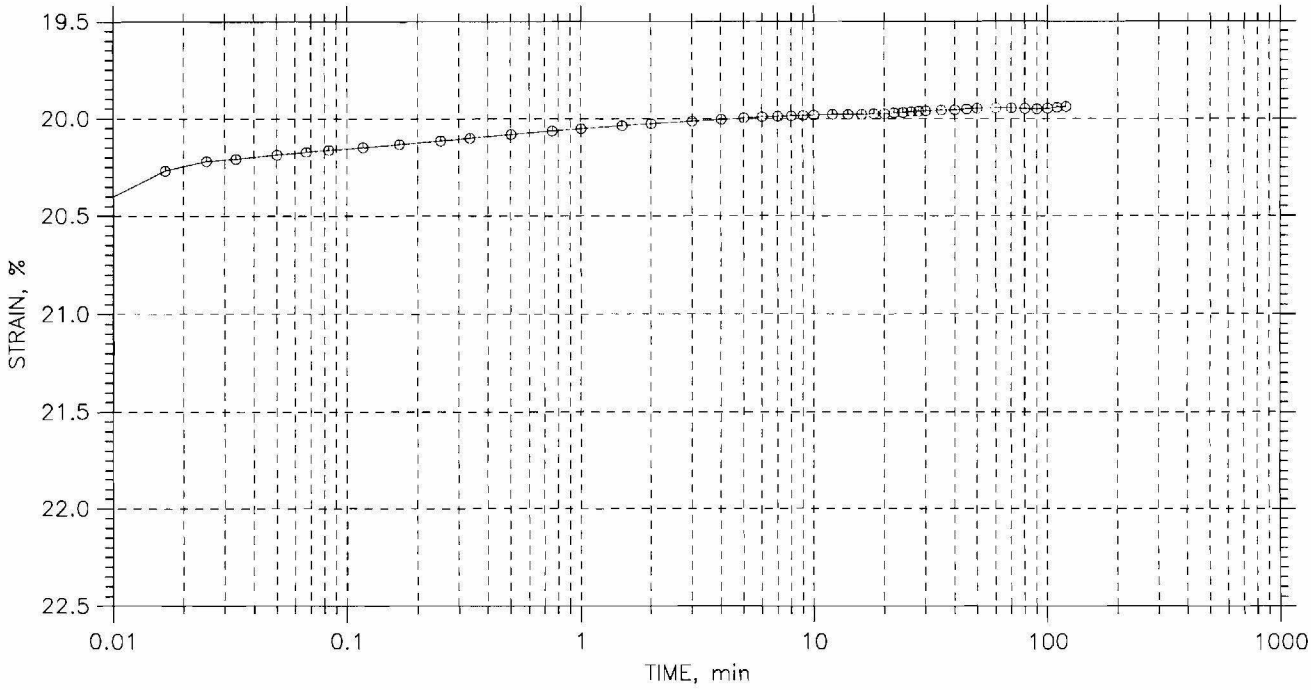
Wed, 14-FEB-2007 13:47:06

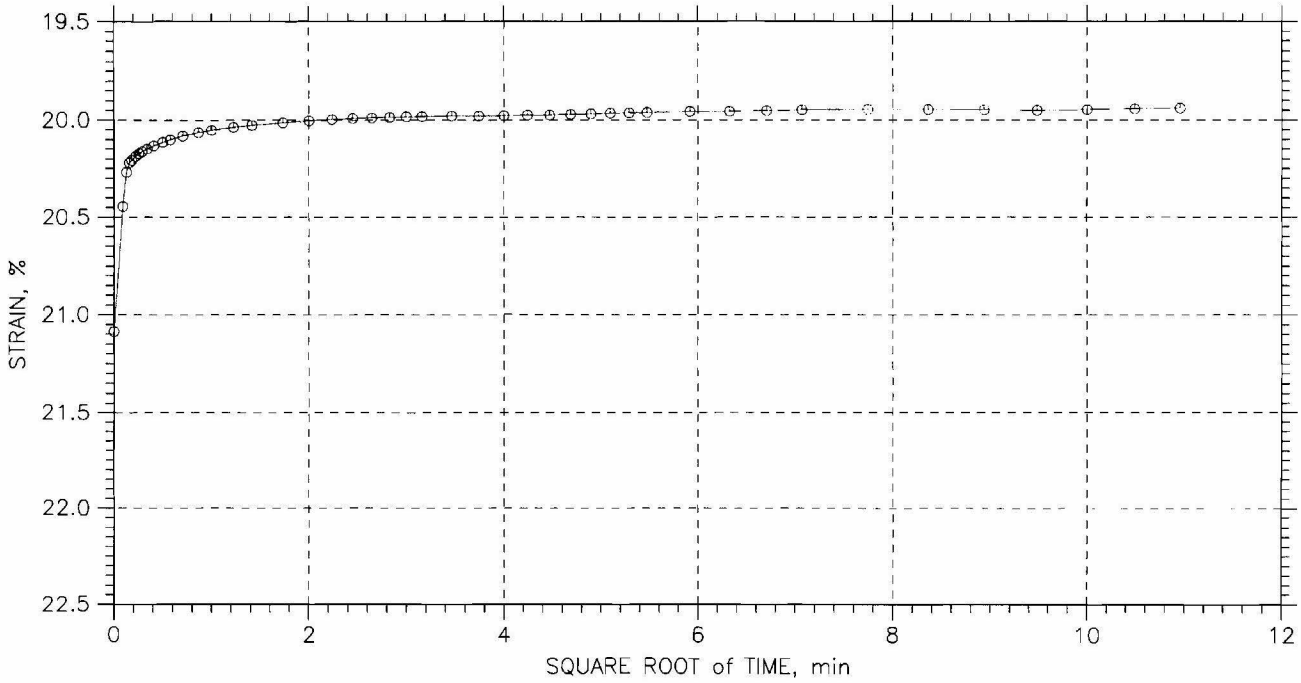
CONSOLIDATION TEST DATA

TIME CURVES

Constant Load Step: 10 of 12

Stress: 8. tsf





GeoTesting express	Project: No. 064006	Location: Windham, ME	Project No.: GTX-7278
	Boring No.: B-114	Tested By: md	Checked By: jdt
	Sample No.: ---	Test Date: 02/06/07	Depth: 23-25 ft
	Test No.: C-1	Sample Type: Tube	Elevation: ---
	Description: Moist, gray clay with traces of sand		
	Remarks: System G		

VIL_RESP00707

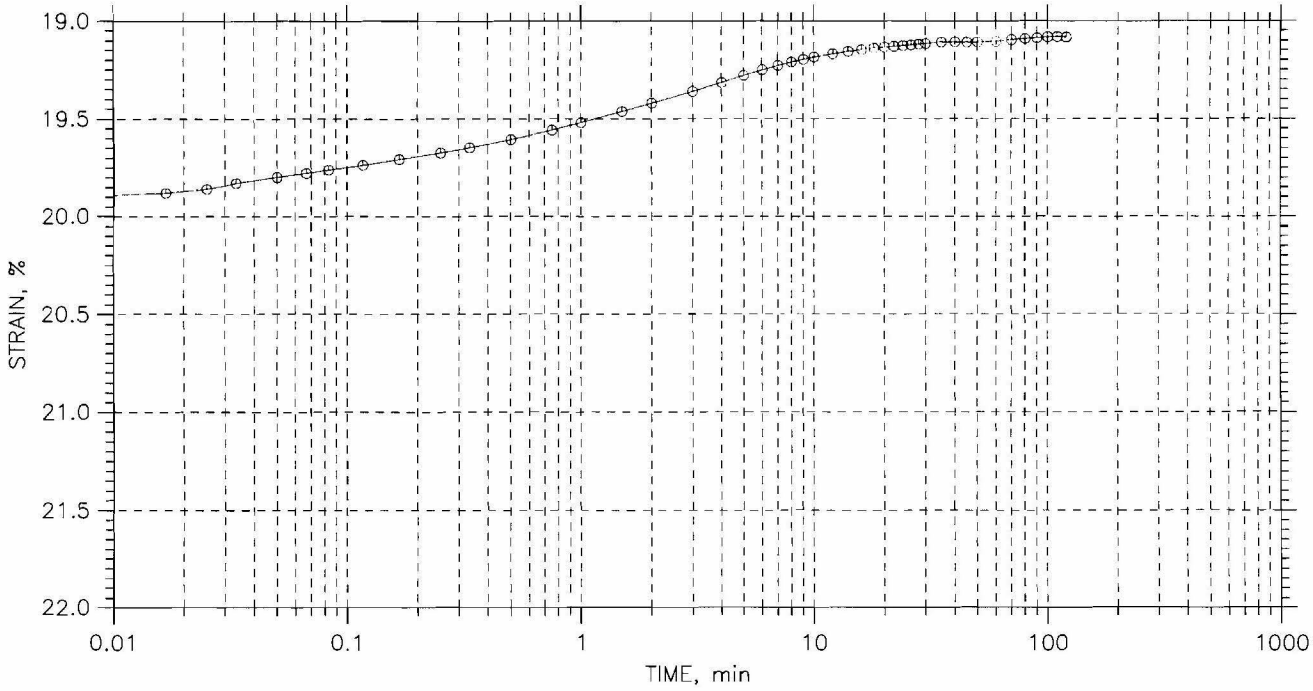
Wed, 14-FEB-2007 13:47:06

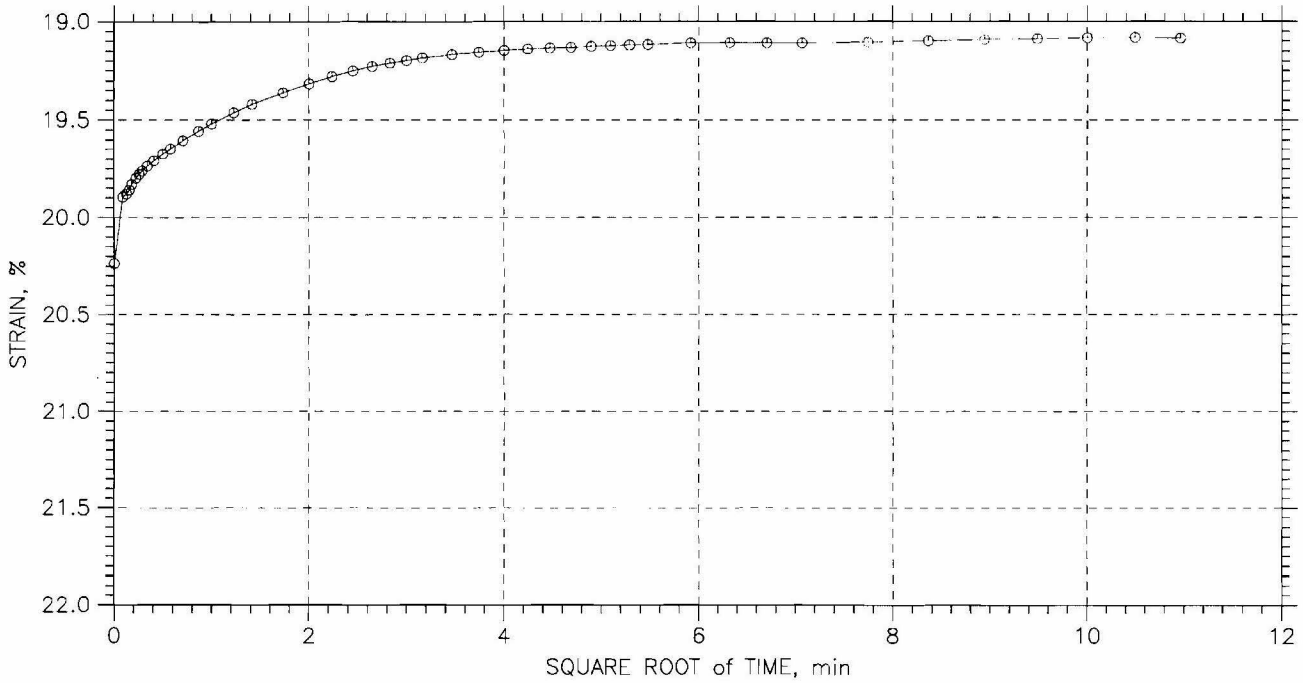
CONSOLIDATION TEST DATA

TIME CURVES

Constant Load Step: 11 of 12

Stress: 2. tsf





GeoTesting express	Project: No. 064006	Location: Windham, ME	Project No.: GTX-7278
	Boring No.: B-114	Tested By: md	Checked By: jdt
	Sample No.: ---	Test Date: 02/06/07	Depth: 23-25 ft
	Test No.: C-1	Sample Type: Tube	Elevation: ---
	Description: Moist, gray clay with traces of sand		
	Remarks: System G		

VIL_RESP00708

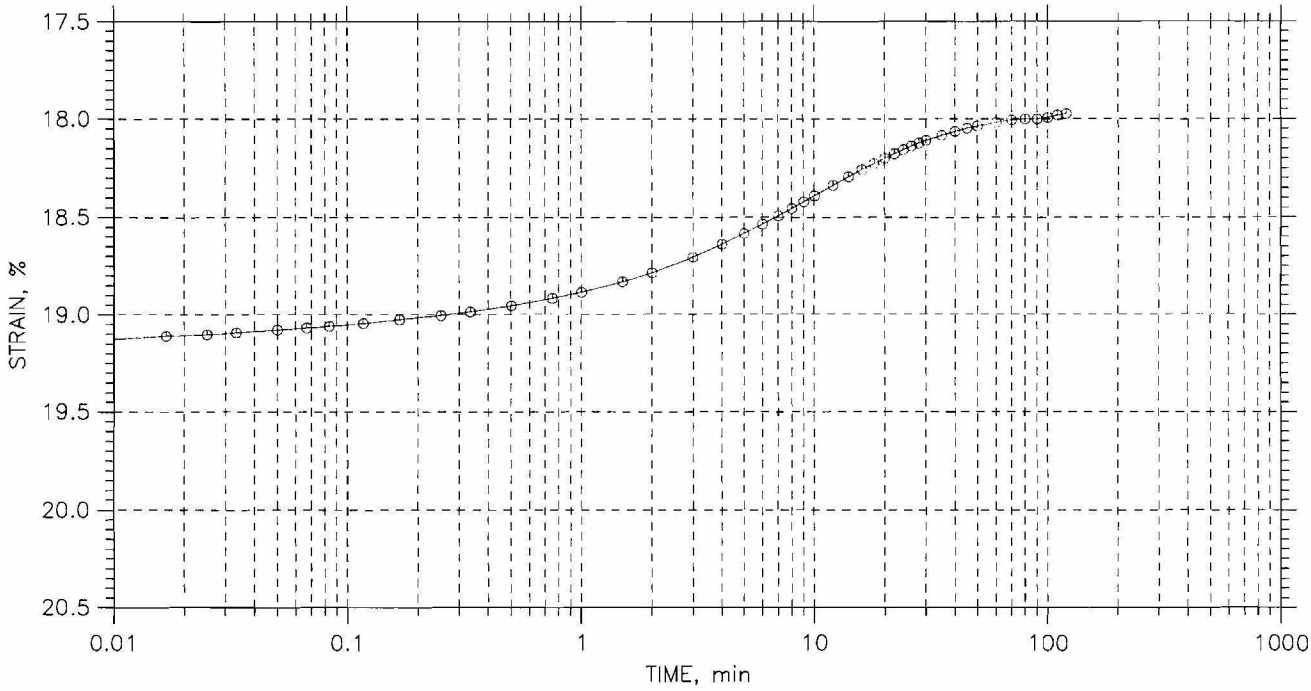
Wed, 14-FEB-2007 13:47:06

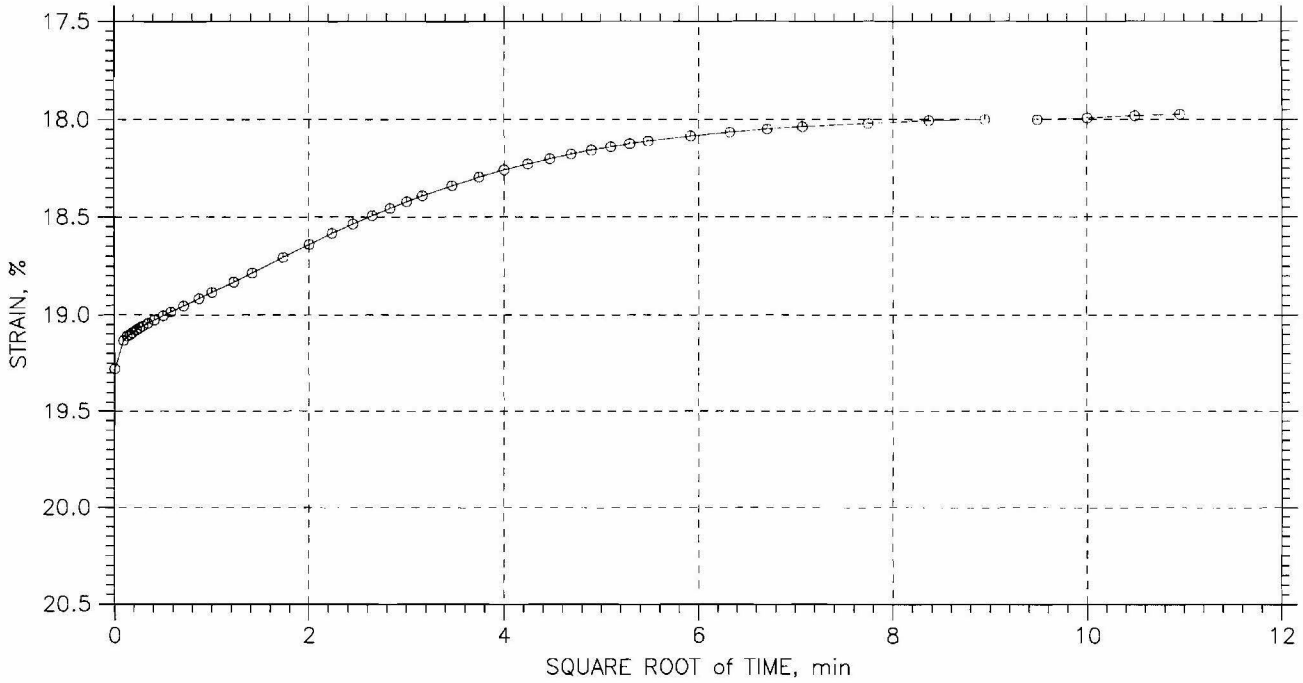
CONSOLIDATION TEST DATA

TIME CURVES

Constant Load Step: 12 of 12

Stress: 0.5 tsf





GeoTesting express	Project: No. 064006	Location: Windham, ME	Project No.: GTX-7278
	Boring No.: B-114	Tested By: md	Checked By: jdt
	Sample No.: ---	Test Date: 02/06/07	Depth: 23-25 ft
	Test No.: C-1	Sample Type: Tube	Elevation: ---
	Description: Moist, gray clay with traces of sand		
	Remarks: System G		

VIL_RESP00709

Wed, 14-FEB-2007 13:47:06

CHAIN OF CUSTODY

#2064353
$200 / 1 month

GeoTesting Express, Inc.
1145 Massachusetts Avenue
Boxborough, MA 01719
(978) 635-0424 Fax (978) 635-0266

Company Name: OAK ENGINEERS
Address: BADGER'S WHARF, NEWBURYPORT, MA 01950
Contact: WENDELL SHEDD
Phone Number: 978-465-9877
Fax Number: 978-465-2986
Project Name: 064006
Project Location: WENDHAM, ME

Sample Type: 1. Soil (circled) 2. Geosynthetic 3. Rock 4. Concrete 5. Other
Container Type: 1. Bucket 2. Bag 3. Jar 4. Tube (circled) 5. Roll

Analysis: CU TEST W/ PORE PRESSURE MEASUREMENTS; D4767 CONSOLIDATION; TEST W/ 1 CYCLE LOAD-UNLOAD 350

Sample Identification	Container Size	Container Type	Sampling Date	Sampling Time	Sample Type	CU TEST W/ PORE PRESSURE MEASUREMENTS	D4767 CONSOLIDATION	TEST W/ 1 CYCLE LOAD-UNLOAD 350				Comments
B114 23'-25'			1/22	11:00 AM	SOIL	X	X					~~20~~ 15 PSI CONFINING PRESSURE

Relinquished By: Wendell Shedd III	Date: 2/1/07 Time: 10:00AM	Received By:	Date: 2/1/07 Time: 1100
Relinquished By:	Date: Time:	Received By:	Date: 2/1/07 Time: 12:00
Relinquished By:	Date: Time:	Received By:	Date: Time:
SHIPPED VIA:			

Special Instructions
Rush _____ number of days____
Standard _____ 10 days

VIL_RESP00710

GeoTesting
express
a subsidiary of Geo-Comp Corporation

WARRANTY and LIABILITY

GeoTesting Express (GTX) warrants that all tests it performs are run in general accordance with the specified test procedures and accepted industry practice. GTX will correct or repeat any test that does not comply with this warranty. GTX has no specific knowledge as to conditioning, origin, sampling procedure or intended use of the material.

GTX may report engineering parameters that require us to interpret the test data. Such parameters are determined using accepted engineering procedures. However, GTX does not warrant that these parameters accurately reflect the true engineering properties of the *in situ* material. Responsibility for interpretation and use of the test data and these parameters for engineering and/or construction purposes rests solely with the user and not with GTX or any of its employees.

GTX's liability will be limited to correcting or repeating a test which fails our warranty. GTX's liability for damages to the Purchaser of testing services for any cause whatsoever shall be limited to the amount GTX received for the testing services. GTX will not be liable for any damages, or for any lost benefits or other consequential damages resulting from the use of these test results, even if GTX has been advised of the possibility of such damages. GTX will not be responsible for any liability of the Purchaser to any third party.

Commonly Used Symbols

A	pore pressure parameter for $\Delta\sigma_1 - \Delta\sigma_3$
B	pore pressure parameter for $\Delta\sigma_3$
CIU	isotropically consolidated undrained triaxial shear test
CR	compression ratio for one dimensional consolidation
C_c	coefficient of curvature, $(D_{30})^2 / (D_{10} \times D_{60})$
C_u	coefficient of uniformity, D_{60}/D_{10}
C_c	compression index for one dimensional consolidation
C_α	coefficient of secondary compression
c_v	coefficient of consolidation
c	cohesion intercept for total stresses
c'	cohesion intercept for effective stresses
D	diameter of specimen
D_{10}	diameter at which 10% of soil is finer
D_{15}	diameter at which 15% of soil is finer
D_{30}	diameter at which 30% of soil is finer
D_{50}	diameter at which 50% of soil is finer
D_{60}	diameter at which 60% of soil is finer
D_{85}	diameter at which 85% of soil is finer
d_{50}	displacement for 50% consolidation
d_{90}	displacement for 90% consolidation
d_{100}	displacement for 100% consolidation
E	Young's modulus
e	void ratio
e_c	void ratio after consolidation
e_o	initial void ratio
G	shear modulus
G_s	specific gravity of soil particles
H	height of specimen
PI	plasticity index
i	gradient
K_o	lateral stress ratio for one dimensional strain
k	permeability
LI	Liquidity Index
m_v	coefficient of volume change
n	porosity
PI	plasticity index
P_c	preconsolidation pressure
p	$(\sigma_1 + \sigma_3) / 2$, $(\sigma_v + \sigma_h) / 2$
p'	$(\sigma'_1 + \sigma'_3) / 2$, $(\sigma'_v + \sigma'_h) / 2$
p'_c	p' at consolidation
Q	quantity of flow
q	$(\sigma_1 - \sigma_3) / 2$
q_f	q at failure
q_o, q_i	initial q
q_c	q at consolidation
S	degree of saturation
SL	shrinkage limit
s_u	undrained shear strength
T	time factor for consolidation
T	temperature
t	time
U, UC	unconfined compression test
UU, Q	unconsolidated undrained triaxial test
u_a	pore gas pressure
u_e	excess pore water pressure
u, u_w	pore water pressure
V	total volume
V_g	volume of gas
V_s	volume of solids
V_v	volume of voids
V_w	volume of water
V_o	initial volume
v	velocity
W	total weight
W_s	weight of solids
W_w	weight of water
w	water content
w_c	water content at consolidation
w_f	final water content
w_l	liquid limit
w_n	natural water content
w_p	plastic limit
w_s	shrinkage limit
w_o, w_i	initial water content
α	slope of q_f versus p_f
α'	slope of q_f versus p_f'
γ_t	total unit weight
γ_d	dry unit weight
γ_s	unit weight of solids
γ_w	unit weight of water
ε	strain
ε_{vol}	volume strain
ε_h, ε_v	horizontal strain, vertical strain
μ	Poisson's ratio, also viscosity
σ	normal stress
σ'	effective normal stress
σ_c, σ'_c	consolidation stress in isotropic stress system
σ_h, σ'_h	horizontal normal stress
σ_v, σ'_v	vertical normal stress
σ_1	major principal stress
σ_2	intermediate principal stress
σ_3	minor principal stress
τ	shear stress
φ	friction angle based on total stresses
φ'	friction angle based on effective stresses
φ'_r	residual friction angle
φ_{ult}	φ for ultimate strength

VIL_RESP00711

NCS

Northeast Civil Solutions
INCORPORATED

153 U.S. Route 1
Scarborough
Maine 04074

tel
207.883.1000
800.882.2227

fax
207.883.1001

June 11, 2007

Mr. Ken Elowe, Director
Bureau of Resource Management
Maine Department of Inland Fisheries and Wildlife
284 State Street Station #41
Augusta, Maine 04333

RE: Village at Little Falls Environmental Project Review Comments

Dear Ken,

Enclosed, please find reduced size copies of the revised planset for the Village at Little Falls residential development. These drawings were revised based upon comments we received in your Comment Review Memorandum addressed to Marybeth Richardson of the Maine Department of Environmental Protection, dated April 23, 2007. The review comments are outlined below; our response to each comment follows in bold.

1. Based on the application, I was unclear exactly where and what is included in the buffer restoration plan? While I believe we are on the same page, there should be a specific sheet that clearly depicts the buffer(s), distances, plantings, etc. **Additional hatching has been added to the grading plans in order to help clarify the restoration area. Please refer to Sheet 26 for additional restoration details and a restoration cross-section.**

2. I am concerned about the project timing and instream/adjacent stream work during the winter months when site conditions cannot be permanently stabilized. No instream work would be allowed from 10-1 to July 1, and extra precautions need to take place from fall to winter in the areas immediately adjacent to the stream resource. **A note prohibiting in-stream restoration between the dates October 1st and July 1st has been added to the Grading Plans, the Bank Restoration Plan, and the Erosion Control Notes Plan. Additional precautions for winter construction are outlined in the Erosion Control Notes on Sheet 24.**

3. I have noticed several loads of sand dumped adjacent to the river within what I had considered to be part of the future stream bank restoration area (along emergency entrance on Sappi property) and there are no erosion control measures. Is this sand related t this project? In any case, it should

not have been dumped in this location, and certainly not without some form of containment. **These sand piles are not part of the proposed development and are not located on the applicant's property; therefore the management of these stock piles are not within the control of the applicant.**

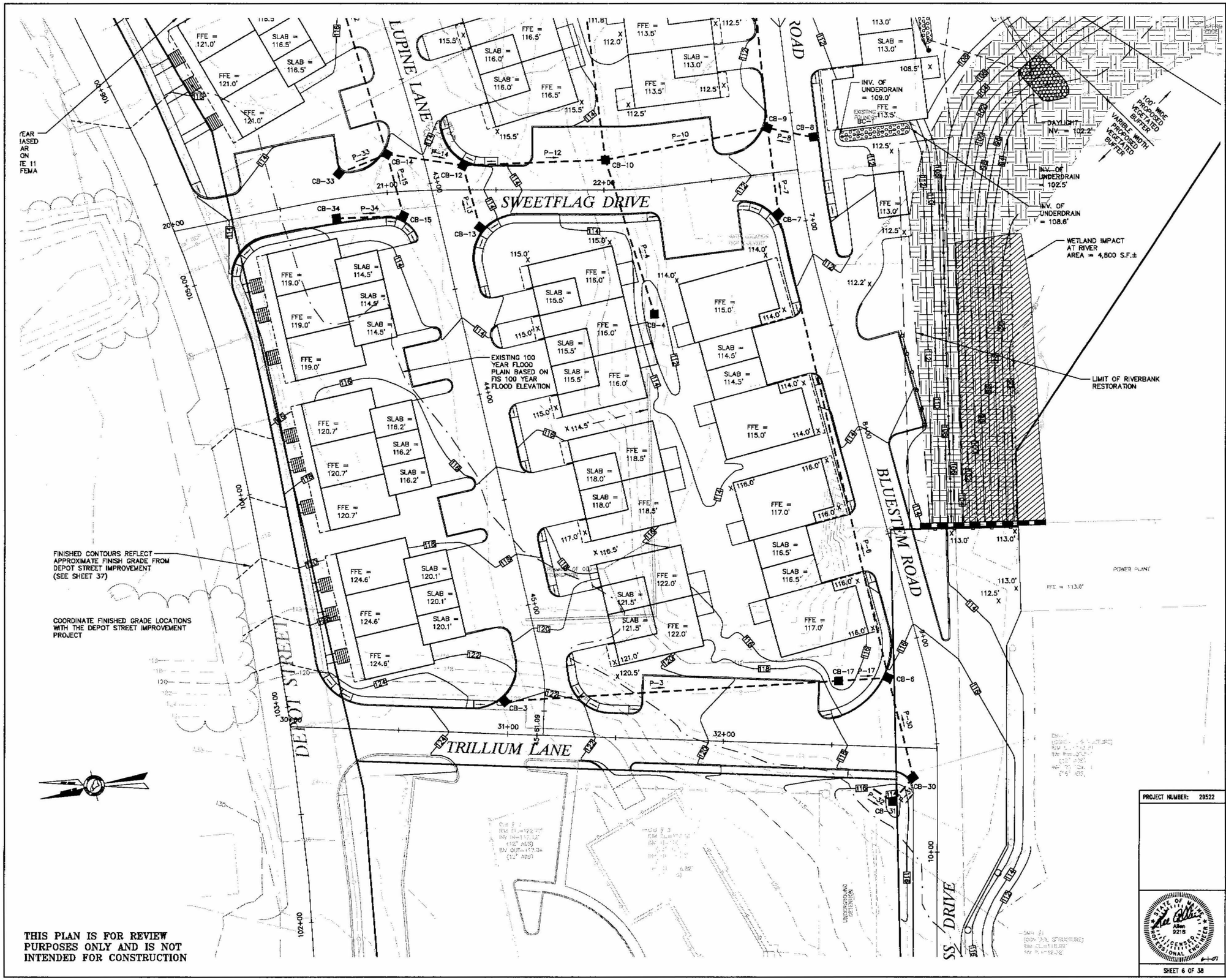
Full size prints with these revisions will be provided to the Maine Department of Environmental Protection in conjunction with the revisions resulting from comments from the Geological Project Review Memorandum. Please feel free to give me a call at 207-883-1000 if you have any questions. Thank you.

Sincerely,
Northeast Civil Solutions, Inc.

Denise Cameron

Denise Cameron, P.E.
Project Engineer

CC: Steve Etzel, HRC-Village at Little Falls, LLC
Marybeth Richardson, Maine Department of Environmental Protection
James Pellerin, Maine Department of Inland Fisheries and Wildlife



THIS PLAN IS FOR REVIEW PURPOSES ONLY AND IS NOT INTENDED FOR CONSTRUCTION

LEGEND

- BOUNDARY LINE
- EDGE OF PAVEMENT
- EXISTING EDGE OF GRAVEL
- RIGHT-OF-WAY LINE
- EDGE OF WETLANDS
- EXISTING EASEMENT
- UTILITY POLE
- EXISTING HYDRANT
- OHU — OVERHEAD UTILITY
- 80 — EXISTING CONTOUR
- EXISTING 100 YEAR FLOOD PLAIN
- 80 — PROPOSED CONTOUR
- 80 — APPROXIMATE CONTOURS OF DEPOT STREET IMPROVEMENT PROJECT BY MDOT & PWD
- VEGETATIVE BUFFER TO RIVER
- PROPOSED RIP RAP
- HANDICAP RAMP (SEE DETAIL)
- PROPOSED LEDGE FACE
- RIVERBANK RESTORATION AREA (SEE DETAIL OF SHEET 26)

NOTES

1. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THESE PLANS, CITY STANDARDS AND SPECIFICATIONS, CURRENT PORTLAND WATER DISTRICT STANDARDS, CENTRAL MAINE POWER REGULATIONS, NFPA 54 NATURAL FUEL GAS CODE, AGA 2002 NATURAL FUEL GAS, AND MDOT STANDARD SPECIFICATIONS, REVISED 2002.
2. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL MAKE CERTAIN THAT ALL PERMITS AND APPROVALS HAVE BEEN OBTAINED. NO CONSTRUCTION OR FABRICATION SHALL BEGIN UNTIL THE CONTRACTOR HAS RECEIVED AND THOROUGHLY REVIEWED ALL PLANS AND OTHER DOCUMENTS APPROVED BY ALL OF THE PERMITTING AUTHORITIES.
3. THE GENERAL CONTRACTOR AND ALL SUB-CONTRACTORS SHALL VERIFY THE SUITABILITY OF ALL EXISTING AND PROPOSED SITE CONDITIONS INCLUDING GRADES AND DIMENSIONS BEFORE COMMENCEMENT OF CONSTRUCTION. THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY OF ANY DISCREPANCIES. MINOR ADJUSTMENTS TO FINISH GRADE TO ACCOMPLISH SPOT DRAINAGE ARE ACCEPTABLE, IF NECESSARY, UPON PRIOR APPROVAL OF ENGINEER. PAVING INSTALLED SHALL "FLUSH OUT" AT ANY JUNCTURE WITH EXISTING PAVING.
4. THE LOCATIONS OF UNDERGROUND UTILITIES SHOWN ON THIS PLAN ARE BASED ON FIELD SURVEYS AND LOCAL UTILITY COMPANY RECORDS. IT SHALL BE THE CONTRACTOR'S FULL RESPONSIBILITY TO CONTACT THE VARIOUS UTILITY COMPANIES TO LOCATE THEIR UTILITIES PRIOR TO STARTING CONSTRUCTION.
5. CONTRACTOR SHALL VERIFY ALL EXISTING UTILITY LOCATIONS, EXISTING INVERTS AND RIM ELEVATIONS PRIOR TO CONSTRUCTION.
6. STORMWATER TREATMENT CALCULATION:

   TOTAL LOT AREA = 349,787 S.F. = 8.03 Ac.

   DISTURBED AREA = 329,030 S.F.

   TOTAL DEVELOPED AREA ONSITE = 318,777 S.F.
   TOTAL DEVELOPED AREA TREATED = 254,821 S.F.
   % AREA TREATED = 80% ≥ 80% REQ'D. = OK

   TOTAL IMPERVIOUS AREA ONSITE = 184,593 S.F.
   TOTAL IMPERVIOUS AREA TREATED = 176,691 S.F.
   % AREA TREATED = 95% ≥ 95% REQ'D. = OK
7. REFER TO THE GEOTECHNICAL INVESTIGATION PERFORMED BY OAK ENGINEERS DATED FEBRUARY 27, 2007 FOR INFORMATION REGARDING FOUNDATIONS, RETAINING WALLS, FILL MATERIAL SPECIFICATIONS, PRE-LOADING REQUIREMENTS, AND ADDITIONAL GEOTECHNICAL CONDITIONS AND SPECIFICATIONS.
8. WETLAND IMPACT:

   740 S.F. OF FRESHWATER WETLAND
   (MAN-MADE DRAINAGE CHANNEL)

   4,800 S.F. OF IMPACT TO PRESUMPSCOT RIVER
   (FOR SHORE RESTORATION)

   TOTAL IMPACT = 5,540 S.F.
9. SEE SHEET 29 FOR BIORETENTION CELL LEGEND AND DETAILS.
10. SEE SHEET 29 FOR PIPE AND STRUCTURE SCHEDULES.
11. ALL BUILDINGS SHALL BE CONSTRUCTED WITH THE F.F.E. A MINIMUM OF 1 FOOT ABOVE 100 YEAR FLOOD PLAIN AND AT AN ELEVATION NO LESS THAN INDICATED ON THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) CONDITIONAL LETTER OF MAP REVISION FOR FILL (CLOMR) DATED MAY 8, 2007 FOR TOWN OF WINDHAM, PANEL NUMBER 23018900259.
12. NO IN-STREAM WORK IS PERMITTED BETWEEN THE DATES OF OCTOBER 1 TO JULY 1.

Revisions:	By:	Date:	Changes:

PROJECT NUMBER: 29522	ACAD FILE: 29522-GRADING.DWG	SCALE: 1" = 20'	DATE: JUNE 1, 2007

Drawing Name:
GRADING AND DRAINAGE PLAN – SHEET 2

Project Name and Location:
VILLAGE AT LITTLE FALLS
ROUTE 202, WINDHAM, MAINE

Prepared For:
HRC-VILLAGE AT LITTLE FALLS, LLC
2 MARKET STREET, PORTLAND, MAINE 04101

STATE OF MAINE — Alan Allen 9218 — LICENSED PROFESSIONAL ENGINEER

SURVEYING ENGINEERING LAND PLANNING
Northeast Civil Solutions
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153 US ROUTE 1, SCARBOROUGH, MAINE 04074
tel 207.883.1000 800.882.2227
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e-mail info@northeastcivilsolutions.com

0 20' 40' 80'

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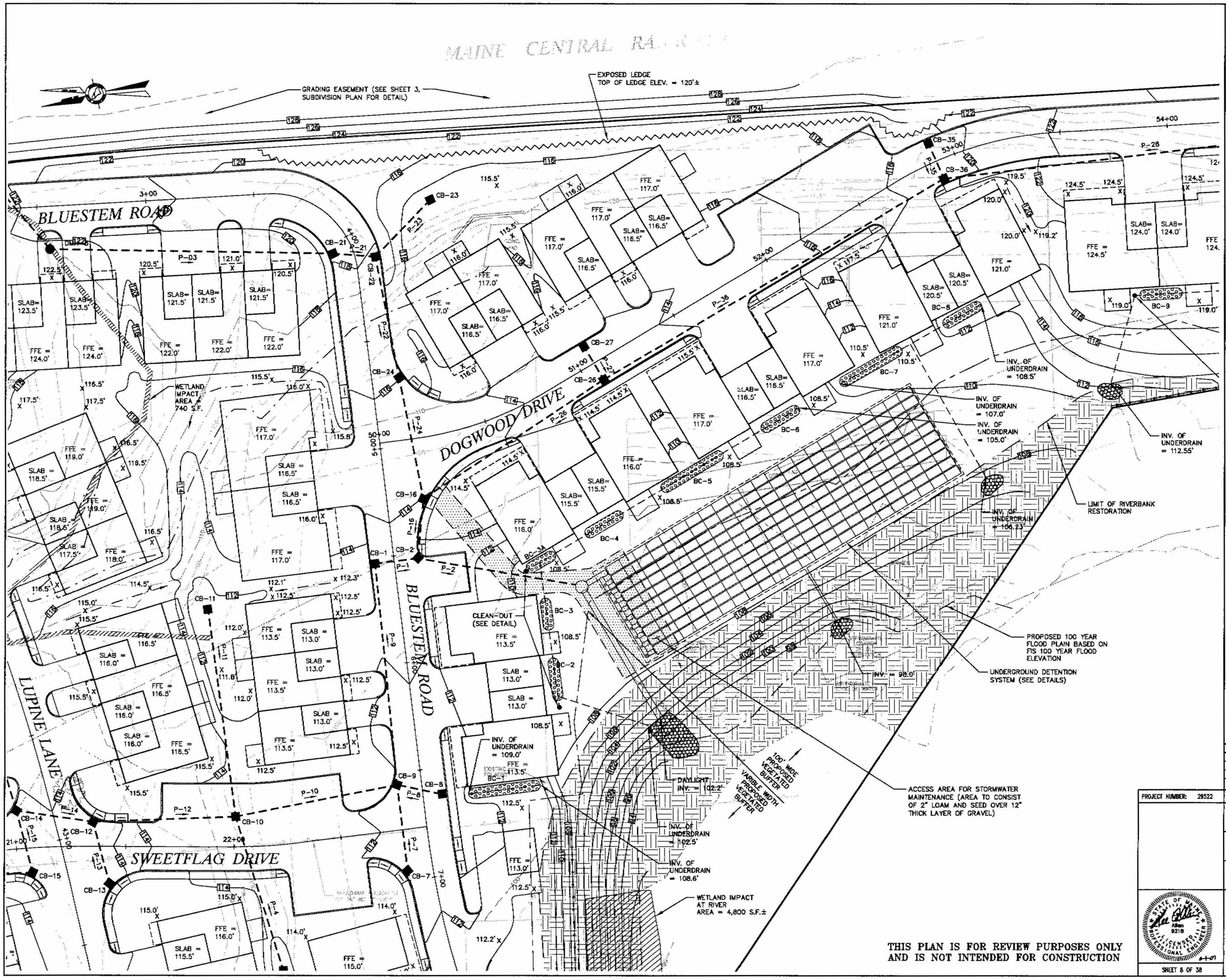

THIS PLAN IS FOR REVIEW PURPOSES ONLY AND IS NOT INTENDED FOR CONSTRUCTION

LEGEND

- BOUNDARY LINE
- EDGE OF PAVEMENT
- EXISTING EDGE OF GRAVEL
- RIGHT-OF-WAY LINE
- EDGE OF WETLANDS
- EXISTING EASEMENT
- UTILITY POLE
- EXISTING HYDRANT
- OHU — OVERHEAD UTILITY
- 80 — EXISTING CONTOUR
- EXISTING 100 YEAR FLOOD PLAIN
- 80 — PROPOSED CONTOUR
- 80 — APPROXIMATE CONTOURS OF DEPOT STREET IMPROVEMENT PROJECT BY MDOT & PWD
- VEGETATIVE BUFFER TO RIVER
- PROPOSED RIP RAP
- HANDICAP RAMP (SEE DETAIL)
- PROPOSED LEDGE FACE
- RIVERBANK RESTORATION AREA (SEE DETAIL OF SHEET 26)

NOTES

1. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THESE PLANS, CITY STANDARDS AND SPECIFICATIONS, CURRENT PORTLAND WATER DISTRICT STANDARDS, CENTRAL MAINE POWER REGULATIONS, NFPA 54 NATURAL FUEL GAS CODE, AGA 2002 NATURAL FUEL GAS, AND MDOT STANDARD SPECIFICATIONS, REVISED 2002.
2. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL MAKE CERTAIN THAT ALL PERMITS AND APPROVALS HAVE BEEN OBTAINED. NO CONSTRUCTION OR FABRICATION SHALL BEGIN UNTIL THE CONTRACTOR HAS RECEIVED AND THOROUGHLY REVIEWED ALL PLANS AND OTHER DOCUMENTS APPROVED BY ALL OF THE PERMITTING AUTHORITIES.
3. THE GENERAL CONTRACTOR AND ALL SUB-CONTRACTORS SHALL VERIFY THE SUITABILITY OF ALL EXISTING AND PROPOSED SITE CONDITIONS INCLUDING GRADES AND DIMENSIONS BEFORE COMMENCEMENT OF CONSTRUCTION. THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY OF ANY DISCREPANCIES. MINOR ADJUSTMENTS TO FINISH GRADE TO ACCOMPLISH SPOT DRAINAGE ARE ACCEPTABLE, IF NECESSARY, UPON PRIOR APPROVAL OF ENGINEER. PAVING INSTALLED SHALL "FLUSH OUT" AT ANY JUNCTURE WITH EXISTING PAVING.
4. THE LOCATIONS OF UNDERGROUND UTILITIES SHOWN ON THIS PLAN ARE BASED ON FIELD SURVEYS AND LOCAL UTILITY COMPANY RECORDS. IT SHALL BE THE CONTRACTOR'S FULL RESPONSIBILITY TO CONTACT THE VARIOUS UTILITY COMPANIES TO LOCATE THEIR UTILITIES PRIOR TO STARTING CONSTRUCTION.
5. CONTRACTOR SHALL VERIFY ALL EXISTING UTILITY LOCATIONS, EXISTING INVERTS AND RIM ELEVATIONS PRIOR TO CONSTRUCTION.
6. STORMWATER TREATMENT CALCULATION:

   TOTAL LOT AREA = 349,787 S.F. = 8.03 Ac.

   DISTURBED AREA = 329,030 S.F.

   TOTAL DEVELOPED AREA ONSITE = 318,777 S.F.
   TOTAL DEVELOPED AREA TREATED = 254,621 S.F.
   % AREA TREATED = 80% $\geq$ 80% REQ'D. = OK

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Revisions:	By:	Date:	Changes:

PROJECT NUMBER: 29522	ACAD FILE: 29522-GRADING.DWG	SCALE: 1" = 20'	DATE: JUNE 1, 2007

Drawing Name:
GRADING AND DRAINAGE PLAN – SHEET 4

Project Name and Location:
VILLAGE AT LITTLE FALLS
ROUTE 202, WINDHAM, MAINE

Prepared For:
HRC-VILLAGE AT LITTLE FALLS, LLC
2 MARKET STREET, PORTLAND, MAINE 04101

STATE OF MAINE — Allen 9218 — LICENSED PROFESSIONAL ENGINEER

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0 20' 40' 80'

SHEET 8 OF 38

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